

Memo

TO:	Josie Tayse, Final Design Project Manager Metro District, Waters Edge
FROM:	Paul Martin, Assistant Foundations Engineer Geotechnical Engineering Section
Concur:	Rich Lamb, Foundations Design Build Engineer Geotechnical Engineering Section
DATE:	December 15, 2017
SUBJECT:	SP 8825-652, Metrowide Overhead Sign Structure Replacement Subsurface Evaluation and Foundation Recommendation Report

1.0 Project Summary

This report is provided in response to a request by the Metro District to provide a subsurface investigation and foundation recommendations for eight proposed new overhead sign structures being constructed along TH 7, TH 36, TH 47, TH 94 (two sites), TH 100 (two sites) and TH 280. The sign structure types will include both Sign Bridge and Cantilever. Each sign structure will be supported by either spread footings or drilled shaft foundations.



2.0 Subsurface Investigation

The soils at the proposed sign locations were investigated using Standard Penetration Test Borings (SPT) or Cone Penetration Test (CPT) Soundings in October, November and December, 2017. The CPT soundings were interpreted for general soil behavior type and estimated water table elevation. No soil samples were taken so the interpreted soil behavior type may not exactly match what soil is present, but should indicate how it behaves if compared to standard soils.

Based on the results of the investigation we determined that the foundation soils at the proposed sign locations vary; they include loose to very dense sands and layered soft to stiff sandy loam, clay and silt soils. Groundwater was detected in some of the borings and CPT soundings and perched water should also be expected in areas where layered soils exist. Please refer to the attached sounding and boring logs for the most complete description of the foundation soils.

Table 1, below, provides general summaries of the soil and groundwater conditions encountered at/near each sign location.

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SP 8825-652 Metro Overhead Signs TH 7, TH 36, TH 47, TH 94, TH 100 and TH 280 December 15, 2017

Boring		Indicated		
or	Location	Groundwater	Summary of Interpreted Soils	Interpreted
Sounding		Depth (feet)	(Depths in feet)	N-Value
			Sandy Loam and Sand to 9'	25-41
	Site 1, TH 7 WB		Clays and Silts to 16.5'	12-19
T01	STA 311+26, 36' Lt	6.9	Sandy Loam and Clay Loam w/Sand & Gravel	27-39
			Frozen soils and Sands to 7'	20-50+
	Site 7, TH 36 EB		Layered Silts, Clays and Sands to about 25'	3-20
C02	Ramp to SB TH 694	*6.5	Sandy Loam and Sands to termination	14-36
	Site 8, TH 47 SB,			
T03	STA 523+79, 43' Rt	8.5	Fine and very fine grained sands	12-85
	Site 8, TH 47 SB,			
T04	STA 523+81, 20' Lt	11.2	Fine sand, very fine sand and loamy sand	3-66
	Site 9, TH 94 WB		Plastic Silt Loam to 10'	15-33
T05	STA 2118+46, 31 Lt	NI-36'	Sands & Gravel to termination.	21-48
			Loamy sand to 4, clay loam to 6.5,	23,12
	Site 10, TH 94 WB,		silt loam to 9, sands to 19,	8,7-23
T06	STA 428+52, 52' Lt	*	sandy clay to 31.5, sandy loam	10-13,9-17
			Sand fill to 11.5, topsoil to 13,	18-41, 14
	Site 10, TH 94 WB,		sand to 21.5, clay to 31.5, over	8-21,6-11
T07	STA 428+55, 28' Rt	*	sandy loam and silty clay	6-22
	Site 11, TH 100 SB		Layered Clays, Sands and Silts to 10.5'	3-32
C08a	STA 183+01, 42'Rt	*	Refusal on dense Sand	50+
	Site 12, TH 100 SB		Sands layered with Silt and Clay to 29.5'	4-50
C09	STA 227+10, 32'Lt	*	Refusal on Dense Sand	30-50+
TC-2	Site 13, TH 280 NB		Plastic Sandy Loam to 13'	11-16
(1969)	near STA 88+60	25	Sand & Gravel to termination.	20-75
*Do	rehad lawara many accur	. NI Nat	Indicated to	

Table 1. Summary of Estimated Soil and Groundwater Conditions

Perched layers may occur

NI- Not indicated to

3.0 Foundation analysis

As part of the overhead sign standard drawings (revised 11-21-2014), standard foundations were developed to support overhead signs. These standard foundations consist of two spread footing and two drilled shaft designs to be used on different sign pole sizes (see Drawing ST-3, Standard Overhead Sign Supports Interim Design B).

- Spread Footings: 12 ½ ft. x 18 ft. or 9 ft. x 14 ft.
- Drilled Shafts: 3 ft. diameter, 23 ft. deep or 4 ft. diameter, 29 ft. deep

In addition, the standard foundation notes state the following requirements:

- All spread footings shall have an allowable design bearing pressure of 1 ¹/₄ tons per square foot
- The drilled shafts have an allowable design lateral • bearing pressure of 250 lbs. per square foot per foot of depth

Based on previous research, these requirements will be met in all but the weakest mineral soils, and therefore represent a very conservative design. In lieu of new foundation designs we checked the foundation soils to see if they met those requirements.





SP 8825-652 Metro Overhead Signs TH 7, TH 36, TH 47, TH 94, TH 100 and TH 280 December 15, 2017

The recommendations contained in the paragraphs below are based on our analysis. We recommend the new foundations consist of drilled shafts designed to meet the requirements of the current standard design tables. At many locations casing used with a head of fluid will be needed to extend the shafts below the groundwater surface. If there are nearby utilities or structures temporary casing is recommended to protect them from ground movement.

Foundation Recommendations

Based on review of the existing subsurface conditions and proposed structures, we recommend:

- The overhead sign structures be supported with drilled shaft foundations as detailed in the typical Foundation Detail Sheet, Drawing ST-3 located on the Traffic Engineering website.
- Table 1 and the attached Overhead Sign Foundation Recommendations Table, showing the estimated soil and water conditions be forwarded along with a copy of the attached Boring and Sounding Logs to the bidding contractors.



- That excavations extending below groundwater be temporarily cased to prevent caving and undermining of nearby structures and pavements and that the casing be removed as the concrete is placed.
- 4. Temporary casing be used to support excavation sides that are near structures or existing utilities.

Attachments: Overhead Sign Foundation Recommendations Table Exploration Plan and X-Section Sketches - 8 Sheets Boring Logs T01, T03, T04, T05, T06, T07, TC-2 (1969) Unique Numbers 82770, 82713, 82714, 82769, 82715, 82716 Sounding Logs C02, C08, C08a, C09, Unique Numbers 82817, 82796, 82824, 82818 SPT Index CPT Index

- cc: J. Tayse, Signing Engineer
 - E. Peterson, Metro Signing Design
 - E. Embacher, Construction Engineer
 - M. Waters, Environmental Stewardship
 - B. Skow, Chief Geotechnical Engineer





	SP 8825-652, Various Metro State Highways, Overhead Sign Foundation Recommendations											
Current Plan		Roadway	Direction	New Structure	Exploration	Exploration	Post	Soils	Ground	Recommendations		
	ID	Туре		Station	Points	Locations	Locations		Water			
NEW CANTILEVER , SITE 1, TH 7	MN7-001	RAMP	WB	Ramp to NB US Baker Road, STA 311+25, 110' west of existing sign	T01	311+26, 36'Lt	SHOULDER	Sandy Loam and Sand to 9', Clays and Silts to 16.5', then Sandy Loam and Clay Loam with layers of Sand & Gravel.	6.9'	Suitable for Standard Design, but temporary casing with a head of fluid should be used to extend the shaft into groundwater.		
NEW CANTILEVER, SITE 7, TH 36	MN36-090	RAMP	EB	Ramp to SB TH 694, STA 61+45, 25' east of existing sign	C02	61+52, 26' Rt	SHOULDER	Frozen soils and Sands to 7', layered Silts, Clays and Sands to about 25', Sandy Loam and Sands to 36'	*6.5	Suitable for Standard Design, but temporary casing with a head of fluid should be used to extend the shaft into groundwater.		
NEW SIGN BRIDGE, SITE 8 TH 47	MN47-004	MAINLINE	SB	TH 47 STA 523+75, 50' south of existing sign	Т03 Т04	523+79, 43'Rt 523+81, 20'Lt	SHOULDER MEDIAN	Fine and very fine Sands	8.5 11.2	Suitable for Standard Design, but temporary casing with a head of fluid should be used to extend the shafts into groundwater.		
NEW CANTILEVER, SITE 9, TH 94	194-688	MAINLINE	WB	2118+30, 30' southeast of existing sign	T05	2118+46, 31' Lt	SHOULDER	Silt Loam to 10', Sandy Loam to 11.5' and Sand & Gravel to 36'		Suitable for Standard Design		
NEW SIGN BRIDGE, SITE 10, TH 94	194-689	MAINLINE	WB	428+50, 250' south of existing sign	Т06 Т07	428+52, 52'Lt 428+55, 28' Rt	MEDIAN SHOULDER	Sands with Clay and Silt layers.	Below 810	Suitable for Standard Design		
NEW CANTILEVER, SITE 11, TH 100	MN100-142		NB	182+80, 30' north of existing sign	C08a C08	183+01, 42'Rt 183+83, 40'Rt	SHOULDER	Layered Clays, Sands and Silts to 10.5'. Refusal on dense Sand	*	Suitable for Standard Design, but temporary casing with a head of fluid should be used to extend the shafts into groundwater.		
NEW CANTILEVER, SITE 12, TH 100	MN100-143	MAINLINE	SB	227+13, 25' South of existing sign bridge	C09	227+10, 32'Lt	SHOULDER	Sands with Clay and Silt layers to 29.5'. Refusal on dense Sand	*	Suitable for Standard Design, but temporary casing with a head of fluid might be needed to extend the shaft into groundwater.		
NEW CANTILEVER, SITE 13, TH 280	MN280-023	MAINLINE	NB	88+60, 25' south of existing sign	TC-2 (1969)	NEAR 88+60	SHOULDER	Sandy Loam to 13', then Sand & Gravel to 34'	25	Suitable for Standard Design, but temporary casing with a head of fluid should be used to extend the shaft into groundwater.		
									*= Perched gr temporary cas	oundwater layers may occur and may require the use of sing.		



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960	48 Loamy Sand and Gravel,	915
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815	13 11	and and Gravel, browns, moist o wet, medium dense to loose			11	Medium-Grained	Sand, brown,
810	10 10	andy Clay with a few bebbles, gray, moist, stiff			10	Sandy Clay with	a few pebbles,
205	9 × p	lastic Sandy Loam, brown,	<u>.</u>		6 * * 7 * *	plastic Sandy Lo	am,
<u></u>	······································				10 22	Silty Clay, brown stiff to very stif	n, moist, f
							OH I94-689
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						34'-0"		
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870	1.6. I Burn plastic Sandy Loam, brown,	870
865	$\begin{array}{c c} 16 \\ 72 \end{array} \xrightarrow{\text{moist, sint to very sint}} \end{array}$	865
	24	
860	20	860
	21 ABB	
855	22 Sand & Gravel, light brown,	855
850	36 27 moist to saturated,	850
	39 medium dense to dense	
845	30.	845
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	APPROXIMATE SOIL BORING LOCATION OH MN280-023	
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LABORATORY LOG & TEST RESULTS - SUBSURFACE EXPLORATION

State Project 8825-652			Bridge No. or Job Desc. OH SIGN MN7-001	Trunk Highway/Location Various Metro	1			Boring N	Vo.		Ground Elevation 1004.4 (DTM)			
Locatio	^{on} Ram	nsey C	County Coordinate System		Drill	Machine	211304	4 CME F	at Tire		SHEET 1 of 1			
	X=4	4843	30 Y=149353 _{'TH 7 WB,}	STA 311+26, 36' Lt	Harr	mer CN	IE Autor	natic Ca	librated		Completed 11/15/17			
	Latit	ude (l	North)=44°55'35.22" Long	tude (West)=93°26'37.79"		SPT	МС	сон	γ	1	Other Tests			
	Depth	26			5	N 60	(%)	(psf)	(pcf)	Sol	Or Remarks			
1Ld		holo			ing ratic	REC	RQD	ACL	Core	×	Formation			
Đ	Elev.	Ľ	Cla	ssification	Drill	(%)	(%)	(ft)	Breaks	Ro	or Member			
-	- -	× · . · . · .× · . · .× · . · .×	slightly plastic Sandy Loam wit	n pebbles, brown, moist, medium		- 25 -	- - - 11							
5-	-	× .	dense to dense		R	41 -	8			a lit	ttle rough drilling 3.5', 7'			
	6.5				सि	-	-							
-	- 9.0		Loamy Sand, gray-brown, satu	rated, medium dense		25	15							
10-	_ 995.4 _					13	42							
-	-		Clay, brown, wet, stiff Sandy Clay Loam, browns with IO			- 12 -	34							
15-	_ 15.0 _ 989.4 _ 16.5			IOS, wet, very stiff	PD -	19 + 19	19			rou	ah drilling at 15.8'			
-	- 987.9 - 17.5 - 986.9	• · • .	Sandy Clay Loa Silty Clay Loa Construction Sandy Clay Loa Sandy Clay Loa	Silty Clay Loam, browns with IC Loamy Sand and Gravel, brow	DS, wet, very stiff ns with IOS, moist, medium		25	- - 33 - 10				g		
20-	19.0 985.4			, °,	, 'o,	· · · · ·	dense Sandy Clay Loam with peb	Sandy Clay Loam with pebbles	, brown, wet, very stiff	- PD	27	20		
-	- 982.9 -	× . • . • .× • . • .×			PD	39	17			rou	gh drilling at 21.5'			
25-	-	· . · .× ·× · . · . · .×			PD	37	- 11							
-	-	× . · . · .× · . · .×	olightly plantic Candy Learn wit	a four poblics and a score of	PD	34	14							
-	-	× . ·	Loamy Sand and Gravel at 23.	5', browns, wet, dense	PD	-	-							
30-	-	· · · · · · · · · · · · · · · · · · ·			PD	35	13							
-	-	· . · .× ·× · . · . · .×				34 .	10							
35-	-	`× ` . ` . ` .×				31 -	11							
	968.4	<u> '~ '</u>	Bottom of Hole - 36.0' Water measured at 6.9' with a	ıger			<u> </u>			I				
				Field Crew	Chief.	O'Donn G:\GIN	el, Pat S	oil Class	:JAH Roc 8825-652-ME	k Cla	ass: Edit: Date: 12/12/17 DISTRICT-OVERHEAD SIGNS.GPJ			



LABORATORY LOG & TEST RESULTS - SUBSURFACE EXPLORATION

State F 8825	Project -652		Bridge No. or Job Desc. Trunk Highway/Location OH SIGN MN47-004 SB Various Metro				Boring I T03	No.		Ground Elevation 894.3 (DTM)
Locatio	^{on} Ram	nsey (County Coordinate System	Drill	Machine	9 21130	4 CME F	at Tire		SHEET 1 of 1
	X=	5289	15 Y=229792 TH 47 SB, STA 523+79, 43'Rt	Han	nmer CN	IE Auto	matic Ca	librated		Drilling 10/24/17 Completed
	Latit	ude (l	North)=45°08'49.28" Longitude (West)=93°16'16.59"		SPT	МС	сон	γ	11	Other Tests
т	Depth	gy		5	N60	(%)	(psf)	(pcf)	So	Or Remarks
DEPTI	Elev.	Litholo	Classification	Drilling Operatic	REC (%)	RQD (%)	ACL (ff)	Core Breaks	Rock	Formation or Member
5- 	- - - - - - - -		Fine Sand, brown with IOS at 8', moist to saturated, medium dense to dense	THAN THAN THAN THAN THAN THAN THAN THAN	18 12 13 27	9 17 21 20				
	- - _ 14.0				41	19				
15-	- ^{880.3} - 16.5		Very Fine Sand, brown, saturated, dense		33	25				
20	- 077.0 - - - - - - -		Fine Sand, browns, saturated, medium dense to very dense		29 15 22 67	24 24 17 22				
- 30 - -	- - -				85	21				
35-	- - - 36 0			PD	73	20				
1	858.3	<u>1, , </u>	Bottom of Hole - 36.0' Water measured at 8.5' with auger		<u>.</u>	<u>L</u>	1	1	<u> </u>	
-				w Chief	: O'Donr G:\GIN	nel, Pat S	Soil Class	:JAH Roc 8825-652-ME	ck Ci TRO	lass: Edit: Date: 12/12/17 DISTRICT-OVERHEAD SIGNS.GPJ



LABORATORY LOG & TEST RESULTS - SUBSURFACE EXPLORATION

State Project 8825-652			Bridge No. or Job Desc. Trunk Highway/Location OH SIGN MN47-004 NB Various Metro				Boring I T04	Vo.		Ground Elevation 896.9 (DTM)
Locatio	ⁿ Ram	isey (County Coordinate System	Drill	Machine	9 21130	4 CME F	at Tire		SHEET 1 of 1
	X=5	5289	74 Y=229815 TH 47 SB, STA 523+81, 20'Lt	Hammer CME			matic Ca	librated		Completed 10/25/17
	Latit	ude (l	North)=45°08'49.51" Longitude (West)=93°16'15.76"		SPT	мс	сон	γ	_	Other Tests
-	Depth	у		Ę	N60	(%)	(psf)	(pcf)	Soi	Or Remarks
μL		holo		ng ratio	REC	ROD	ACL	Core	×	Formation
DE	Elev.	Lit	Classification	Drill	(%)	(%)	(ft)	Breaks	Roc	or Member
	_			R		_				
-	-		Fine Sand, brown, moist, loose	1		+				
-	4.0	· · · ·		X	7.	7				
_	4.0 892.9			11		-				
5-	-	· · ·	Loamy Fine Sand, dark-brown, moist, very loose		3	12				
-	6.5 - 890.4	· ·		11		-				
-	-			X	24 .	_ 11				
-	-	· · · · · ·	Fine Sand, browns, moist to saturated, medium dense to dense	R		+				
1 0−	-			$ \times $	31 -	17				
	11.5 - 885.4	· · · ·		-27]	-				
-	-			$ \times$	19	26				
-	-		Very Fine Sand, gray-brown, saturated, medium dense].	+				
15-	_	· · · · · ·		$\left \right\rangle$	25 -	24				
-	- 16.5 880 4	· · · ·		- PD		†				
	- 000.4	· · · · · ·		\mathbf{X}	22	24				
-	-			PD		+				
20-	-			\square	29 -	24				
-	-	· · · · · ·		PD		+				
	-			\square	29	25				
-	-	· · · · · · ·		PD		+				
25-	_			$\mathbf{\nabla}$	15 -	24				
-	-	· · · · · ·	Fine Sand, browns, saturated, medium dense to very dense	PD		ł				
	-			$\mathbf{\nabla}$	42	22				
	-			(PD		Ļ				
30-	-			$\mathbf{\nabla}$	56	23				
-	-	· · · · · ·		K PD		+				
-	-			\bigvee	50	22				
	-					[<u></u>				
35-	-				66 -	23				
	<u>36.0</u> 860.9		Bottom of Hole - 36 0'	\vee		20				
	000.0		Water measured at 11.2' with auger							
-				Chief	: O'Donr	el. Pat S	Soil Class		ck C	
				0.1101	G:\GIN	TVPROJEC	TS-ACTIVE	8825-652-MI	ETRC	DISTRICT-OVERHEAD SIGNS.GPJ



LABORATORY LOG & TEST RESULTS - SUBSURFACE EXPLORATION

State I 8825	State Project 8825-652		Bridge No. or Job Desc. OH SIGN I-94-688	Trunk Highway/Location Various Metro			Boring No. T05		(Ground Elevation 935.3 (DTM)	
Locatio	on Ram	nsey (County Coordinate System		Drill Machine 211304 CME			4 CME F	at Tire		SHEET 1 of 1
	X=482615 Y=211587 TH 94 EB STA 2118+46, 31' Lt			Han	nmer CN	IE Auto	matic Ca	librated		Completed 11/14/17	
	Latitude (North)=45°05'49.67" Longitude (West)=93°27'02.34"					SPT	MC	СОН	γ	ii -	Other Tests
н	Depth	Лbc			ы	N60	(%)	(psf)	(pcf)	S.	Or Remarks
EP1	·····	ithol		acification	lling erati	REC	RQD	ACL	Core	с Х	Formation
D	Elev.	-	Cia	ssincation		(%)	(%)	(ft)	Breaks	Ř	or Member
-	-				Ę		+				
-	-					15	18				
-	-				िसि		Į				
5-	-		slightly plastic Silt Loam, browr	ns, moist, stiff to very stiff		25	19				
-	-				िरि		t				
-	-					33	15				
-	40.0				R]	+				
10-	925.3	× .	slightly plastic Sandy Loam wit	h some Gravel, brown, moist,	\neg	31	13				
-	11.5 923.8	0, 1	very stiff		-2]	Į			a litt	le rougher drilling 11.5' to
-	-	, `°, 0,				40	8			36'	
-	-	· o' o '			<u></u>	-	+				
15-	-	, o , o	Loamy Sand and Gravel, dark-	browns, moist, dense		35	6				
-	-	, ´o ,			_₹₹		+				
-	10.0	· 0'			X	48	4				
20-	916.3	0 0 (-47		<u> </u>				
- 20	-	, , , , , ,			\square	20	2				
-	-	ò, `			F		+				
-	-	°, °				24	+ 2				
25-		, °, 0,				20 -	÷ 1				
-	-	· o' oʻ			F	29	+ '				
-	-	, o 0 , (Sand and Gravel, brown, damp	to dry, dense		31	1				
	-	, 'o ,			सि		Ţ				
30-	_	· • •				30	+ 1				
-	-	ا`ه `			रि		+				
	-	, `o`,				39	1				
-	-	· •			নি		+				
35 -	36.0	, , , , , ,				39 -	+ 1				
	899.3		Bottom of Hole - 36.0'		\	4	1	I	1	1	
			No water encountered or meas	ured during drilling							
				Field Cre	ew Chief	: O'Doni G:\GlA	nel, Pat S	Soil Class	:JAH Roc 3825-652-ME	k Cla	ss: Edit: Date: 12/12/17 ISTRICT-OVERHEAD SIGNS.GPJ
											· · · · · · · · · · · · · · · · · · ·



LABORATORY LOG & TEST RESULTS - SUBSURFACE EXPLORATION

State Project 8825-652			Bridge No. or Job Desc. OH SIGN 194-689	Trunk Highway/Location Various Metro		Boring No. T06			Ground Elevation 841.2 (DTM)		
Locatio	Location Ramsey County Coordinate System			Drill Machine 211304 CME Fat			at Tire		SHEET 1 of 1		
	X=525191 Y=199719 TH 94 WB, STA 428+52, 52'Lt			Han	nmer CN	IE Autor	natic Ca	librated		Drilling Completed 10/26/17	
1	Latit Depth	ude (l	North)=45°03'52.41" Longi	tude (West)=93°17'09.05"		SPT N60	MC (%)	COH (psf)	γ (pcf)	Soil	Other Tests Or Remarks
DEPTH	Elev.	Litholog	Clas	ssification	Drilling Operatio	REC (%)	RQD (%)	ACL	Core Breaks	Rock	Formation or Member
-	- - - 4.0	· · · · · · · · · · · · · · · · · · ·	Loamy Sand with a little Gravel, dark-brown, moist, medium dense			23	- 7				
5-	837.2 6.5		Clay Loam, brown with IOS, m	bist, stiff		12	15				
-	- 834.7 9.0		slightly organic slightly plastic Silt Loam, dark-brown, moist, firm			8.	19				
10 <i>-</i> -	832.2 11.5		Medium-Grained Sand with a little Gravel, brown, moist, medium dense			13	6				
- - 15 - - -	- 829.7 - - -	0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0, 0	Sand and Gravel, browns, mois	st to wet, medium dense to loose		23 14 7	- 5 - 7 - 8			a li 19	ittle rough drilling 11.5' - '
- 20 - - -	_ 19.0 _ 822.2 - -					11	- - - - - - - - - - - - - - - - - - -				
25-	-		Sandy Clay with a few pebbles, gray, moist, stiff			11	17				
-	-				R	10	15				
30-	- - 31.5 - 809.7	×			R	10	16				
- 35 -	36.0	× · . ·× ·×	plastic Sandy Loam, brown, wet, stiff to very stiff		R	17	14				
	805.2		Bottom of Hole - 36.0' No water encountered or meas	ured during drilling		4	1	1	1	1	



LABORATORY LOG & TEST RESULTS - SUBSURFACE EXPLORATION

8825-652			OH SIGN 194-689	IGN 194-689 Various Metro				T07			842.3 (DTM)	
ocation Ramsey County Coordinate System			Drill Machine 2113			04 CME Fat Tire			SHEET 1 of 1			
X=525270 Y=199704 if a is a result of the set of the				Hammer CME Automatic Calibrated				Drilling Completed 10/26/1				
Latitude (North)=45°03'52.27" Longitude (West)=93°17'07.94"			SPT	МС	сон	γ	ii	Other Tests				
H	Depth	Depth		g Ition	N60	(%)	(psf)	(pcf)	So	Or Remarks		
DEF	Elev.	Litho	Cla	ssification	Drillin	(%)	(%)	ACL (ft)	Core Breaks	Rock	Formation or Member	
-	-		Loamy Sand with a little Grave	l, browns, moist, dense	X	31	6					
5-	- 65	· · · · · · · · · · · · · · · · · · ·				41	7					
+	- 835.8 -	× . · . · .× · . · .×	elightly plastic Sandy Learn wit	h a faw pabblas, brawn, maist		22	12					
10		· . · .× ·× · . · . · .×	very stiff			18	13					
+	- 830.8 - 13.0	· · · · · · · · · · · · · · · · · · ·	organic Loam, black, moist, sti Sand with a seam of organic I	ff pam, brown, moist, medium		14	13					
15	829.3 14.0 828.3		\dense Loamy Fine Sand, dark-brown,	moist, loose		8	14					
-	16.5 825.8		Medium-Grained Sand, brown,	moist, medium dense	E E E E E	11	4					
20-	21.5 820.8				- <u></u> - X	21	5 - - - 18					
25-	- Sondy Clausith a factor	Sandy Clay with a few peobles	es aray moist firm to stiff	R R R	11	- - 17 -						
+	-				R	10	17					
30+	- 31.5 810.8	× .			- <u>R</u>	6	18					
25	-	· · · × ·× · ·	plastic Sandy Loam, brown, we	et, firm	R	6	_ 16 _					
35+	- - 36.5 - 805.8				- <u></u> <u></u>	7	16					
40	-		Silty Clay, brown, moist, stiff to	very stiff	R		19					
	41.0 801.3		Bottom of Hole - 41.0' No water encountered or meas	ured during drilling		22	20					



LABORATORY LOG & TEST RESULTS - SUBSURFACE EXPLORATION

State Project 8825-652			Bridge No. or Job Desc. OH SIGN MN280-023	Trunk Highway/Location Various Metro	1		Boring No. TC-2 (1969)))	Ground Elevation 877.0 (from Plan)		
Locatio	on Ram	nsey C	County Coordinate System		Drill	Drill Machine Soil Sampler				SHEET 1 of 1		
	X= Y= 'SP 6242-41, STA 135+25, 20' Lt			Hammer Rope & Cathead				Completed 4/9/	<u>;9</u>			
	Latitude (North)= Longitude (West)=		_	SPT	MC	СОН	γ	oil	Other Tests			
ΗL	Depth	logy			tion	1160	(%)	(pst)	(pct)	S	Or Remarks	
DEP	Elov	Litho	Cla	ssification	rilling	REC	RQD	ACL	Core	sock	Formation	
	LICV.	х́.			00	(70)	(70)	(11)	LICANS	<u> </u>	or Merriber	
-	2.0	· · · × ·× ·	organic Loam, black, moist			11	1					
-	875.0	× . ×			4		_					
-	-	`× ` . ` . ` .×			X	11	+					
5-	-	'× ' . ' . ' .×			41	-	+					
-	-	× .	alastia Osastalasana harrar		\square	16	+					
-	-	× .	plastic Sandy Loam, brown, mo	moist, stiff to very stiff		16	+					
10 -	-	× .				10	1					
-	-					32	+					
-	- 13.0	× : .]			िसि	02	+					
-	864.0				\square	24	1					
15-	_				सि	-	+					
-	-					20	+					
-	-				R		1					
-	-					21	+					
20-	_				Æ	-	+					
-	-		Sand & Gravel, light brown, moist to saturated, medium dense to dense			22	t					
-	-				R		Į					
	-					36	+					
- 25-	_				47	-	+					
-	-					27	Į					
-	-				5		+					
20	-				\square	39	t					
30-	_				S L	30	Ţ					
-	-					30	+					
-	<u>3</u> 4.0					75	<u>†</u>			St	one in sampler tip.	
	843.0		Bottom of Hole - 34' Water measured at 25' with our	ner								
			walei measureu al 23 Willi au	yei								
					hief: C.	Thomse	en Soil C	lass:Tho	msen Ro	ck C	lass: Edit: Date: 12/1	3/17
						G:\GIN	II\PROJEC	IS-ACTIVE	8825-652-MI	ETRC) DISTRICT-OVERHEAD SIGNS	.GPJ



CONE PENETRATION TEST RESULTS





CONE PENETRATION TEST RESULTS





CONE PENETRATION TEST RESULTS

UNIQUE NUMBER 82824



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CONE PENETRATION TEST RESULTS





Minnesota Department of Transportation Geotechnical Section

BORING LOG DESCRIPTIVE TERMINOLOGY

USER NOTES, ABBREVIATIONS AND DEFINITIONS - Additional information available in Geotechnical Manual

This boring was made by ordinary and conventional methods and with care deemed adequate for the Department's design purposes. Since this boring was not taken to gather information relating to the construction of the project, the data noted in the field and recorded may not necessarily be the same as that which a contractor would desire. While the Department believes that the information as to the conditions and materials reported is accurate, it does not warrant that the information is necessarily complete. This information has been edited or abridged and may not reveal all the information which might be useful or of interest to the contractor. Consequently, the Department will make available at its offices, the field logs relating to this boring.

Since subsurface conditions outside each borehole are unknown, and soil, rock and water conditions cannot be relied upon to be consistent or uniform, no warrant is made that conditions adjacent to this boring will necessarily be the same as or similar to those shown on this log. Furthermore, the Department will not be responsible for any interpretations, assumptions, projections or interpolations made by contractors, or other users of this log.

Water levels recorded on this log should be used with discretion since the use of drilling fluids in borings may seriously distort the true field conditions. Also, water levels in cohesive soils often take extended periods of time to reach equilibrium and thus reflect their true field level. Water levels can be expected to vary both seasonally and yearly. The absence of notations on this log regarding water does not necessarily mean that this boring was dry or that the contractor will not encounter subsurface water during the course of construction.

WATER MEASUREMENT

AB After Bailing
AC After Completion
AFAfter Flushing
w/C with Casing
w/M With Mud
WSDWhile Sampling/Drilling
w /AUG With Hollow Stem Auger
DRILLING OPERATIONS
AUG Augered
CDCore Drilled
DBD Disturbed by Drilling
DBJ Disturbed by Jetting
PDPlug Drilled
ST Split Tube (SPT test)
TW Thinwall (3" Shelby Tube)
WS Wash Sample
AB After Bailing
NSR No Sample Retrieved
WH Weight of Hammer
WR Weight of Rod
Mud Drilling Fluids in Sample
CS Continuous Sample

MISCEL	I ANFOL	JS
MIOOLL		

NA	Not Applicable
w⁄	with
w⁄o	with out
sat	saturated

SOIL CORE TESTS

SPT N60ASTM D1586 N Blows per foot with 140 lb ham standard energy of 210 ft-lbs. represents 60% of the potential system and is the average en by a Rope & Cathead system. MC Moisture Content COHCohesion (equivale Unconfined Compression Streng γ Sample Unit Weight LL Liquid Limit PI Plasticity Index ϕ Angle of Internal Frid REC Percent Core Recov RQD Rock Quality Descr (Percent of total core interval cor unbroken pieces 4 inches or lo ACLAverage Core Leng (Average length of core that is 4 inches long) Core Breaks..... Number of natu breaks per 2 foot interval.

DISCONTINUITY SPACING

FracturesDistanceBeddingVery Close<2 inches</td>Very ThinClose2–12 inchesThinMod. Close12–36 inchesMediumWide>36 inchesThick

DRILLING SYMBOLS



RELATIVE DENSITY

Compactness - Granular Soils

BPF

very loose	0–4
loose	5–10
medium dense	11–24
dense	
very dense	>50
Consistency - Cohesive Soi	
very soft	0_1
soft	2 /
firm	5.8
stiff	
very stiff	16_30
hard	31–60
very bard	×60
very hard	
blk Black	wht White
brn Brown	vel Yellow
orna Orange	It Light
arn Green	dk dark
IOS Iron Ovido Stainod	ar Grov
103 Ifor Oxide Starled	
GRAIN SIZE / DIASTICITY	
VE Very Eine D	Plastic
F Fine slot	Slightly Plastic
Cr Coarse	olightiy i lastic
CI Coarse	
SOIL /BOCK TEBMS	
	mst Limestone
	Sst Sandstone
S Sand [Dolo Dolostone
	weathered
G Gravel (No. 10. Sieve te	
Bldr Boulder (over 2 inches	
T till (upported popetratifie	s uia.) d. alagial dopogita)
1 till (unsolted, honstratille	u glacial deposits)
DOT Triangular Textur	al
lassification System	
assincation system	
CLAY	
100%	
\wedge	
⁵ 90	
80	
60	
50	<u>•</u>
ina ing ing ing ing Alba	Ŷ
	40 T
SiC	
	20 L
TIY Plastic) V	
* & & & &	Å Š.A.
%SILT	<u>`</u> Š
	very loose loose

Minnesota Department of Transportation **Geotechnical Section**



Cone Penetration Test Index Sheet 1.0 (CPT 1.0)

USER NOTES, ABBREVIATIONS AND DEFINITIONS

This Index sheet accompanies Cone Penetration Test Data. Please refer to the Boring Log Descriptive Terminology Sheet for information relevant to conventional boring logs.

This Cone Penetration Test (CPT) Sounding follows ASTM D 5778 and was made by ordinary and conventional methods and with care deemed adequate for the Department's design purposes. Since this sounding was not taken to gather information relating to the construction of the project, the data noted in the field and recorded may not necessarily be the same as that which a contractor would desire. While the Department believes that the information as to the conditions and materials reported is accurate, it does not warrant that the information is necessarily This information has been edited or complete. abridged and may not reveal all the information which might be useful or of interest to the contractor. Consequently, the Department will make available at its offices, the field logs relating to this sounding.

Since subsurface conditions outside each CPT Sounding are unknown, and soil, rock and water conditions cannot be relied upon to be consistent or uniform, no warrant is made that conditions adjacent to this sounding will necessarily be the same as or similar to those shown on this log. Furthermore, the Department will not be responsible for anv interpretations, assumptions, projections or interpolations made by contractors, or other users of this log.

Water pressure measurements and subsequent interpreted water levels shown on this log should be used with discretion since they represent dynamic conditions. Dynamic Pore water pressure measurements may deviate substantially from hydrostatic conditions, especially in cohesive soils. In cohesive soils, water pressures often take extended periods of time to reach equilibrium and thus reflect their true field level. Water levels can be expected to vary both seasonally and yearly. The absence of notations on this log regarding water does not necessarily mean that this boring was dry or that the contractor will not encounter subsurface water during the course of construction.

CPT Terminology

CPT Cone Penetration Test

CPTU......Cone Penetration Test with Pore Pressure measurements

SCPTU......Cone Penetration Test with Pore Pressure and Seismic measurements Piezocone...Common name for CPTU test

(Note: This test is not related to the Dynamic Cone Penetrometer DCP)

qT TIP RESISTANCE

The resistance at the cone corrected for water pressure. Data is from cone with 60 degree apex angle and a 10 cm² end area.

fs SLEEVE FRICTION RESISTANCE

The resistance along the sleeve of the penetrometer.

FR Friction Ratio

Ratio of sleeve friction over corrected tip resistance. FR = fs/qt

V_s Shear Wave Velocity

A measure of the speed at which a siesmic wave travels through soil/rock.

PORE WATER MEASUREMENTS

Pore water measurements reported on CPT Log are representative of water pressures measured at the U2 location, just behind the cone tip, prior to the sleeve, as shown in the figure below. These measurements are considered to be dynamic water pressures due to the local disturbance caused by the cone tip. Dynamic water pressure decay and Static water pressure measurements are reported on a Pore Water Pressure Dissipation Graph.



SBT SOIL BEHAVIOR TYPE

Soil Classification methods for the Cone Penetration Test are based on correlation charts developed from observations of CPT data and conventional borings. Please note that these classification charts are meant to provide a guide to Soil Behavior Type and should not be used to infer a soil classification based on grain size distribution.

The numbers corresponding to different regions on the charts represent the following soil behavior types:

- 1. Sensitive, Fine Grained
- 2. Organic Soils Peats
- 3. Clays Clay to Silty Clay
- 4. Silt Mixtures Clayey Silt to Silty Clay
- 5. Sand Mixtures Silty Sand to Sandy Silt
- 6. Sands Clean Sand to Silty Sand
- Gravelly Sand to Sand 7
- Very Stiff Sand to Clayey Sand 8
- Very Stiff, Fine Grained 9

Note that engineering judgment, and comparison with conventional borings is especially important in the proper interpretation of CPT data in certain geomaterials.

The following charts are used to provide a Soil Behavior Type for the CPT Data.

Robertson CPT 1990

Soil Behavior type based on friction ratio



Robertson CPTU 1990

Soil Behavior type based on pore pressure



where ...

QT	normalized cone resistance
Bq	pore pressure ratio
Fr	Normalized friction ratio
σνο	overburden pressure
σ'νο	effective over burden
pressure	
U2	measured pore pressure
uo	equilibrium pore pressure

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